Proposed Temporary Place of Recreation, Sports or Culture with Ancillary Facilities for a Period of 3 Years and Associated Filling of Land in "Agriculture" Zone, Various Lots in D.D. 112, Shek Kong, Yuen Long, New Territories

Drainage Appraisal

APR 2024

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1. Introduction

1.1 Background

- 1.1.1 The applicant seeks planning permission from the Town Planning Board (the Board) to use Lots 110 S.A RP, 110 S.B, 110 S.C, 110 S.D ss.1 S.A, 110 S.D ss.1 RP, 110 S.D ss.2, 110 S.D ss.3 and 110 S.D RP in D.D. 112, Shek Kong, Yuen Long, New Territories (the Site) for 'Proposed Temporary Place of Recreation, Sports or Culture with Ancillary Facilities for a Period of 3 Years and Associated Filling of Land' (Proposed Development).
- 1.1.2 This Drainage Proposal is to support the planning application for the proposed use.

1.2 The Site

- 1.2.1 The Application Site at Shek Kong has an area of about 2,856 m². It situates Nam Hing West Road and Ko Sheung Road. The site is currently an unused grassland. The site location plan is shown in **Figure 1**.
- 1.2.2 The existing ground level of the site is approx. +26.4 mPD and it is intended to maintain similar site levels in the development. The site and the surrounding are generally flat, the ground levels are similar.
- 1.2.3 There is an existing public 750 mm U Channel by the side of Nam Hing West Road. Existing Drainage Plan is shown in **Figure 2** for reference.
- 1.2.4 There are asbuilt 300mm U Channels (gradient 1 in 100) within the development area. The asbuilt drainage in green solid line are shown in **Figure 3**.
- 1.2.5 Proposed Development Layout plan is shown in **Appendix B** for reference.

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2. Development Proposal

2.1 The Proposed Development

2.1.1 The total site area is approximately 2,856 m². The indicative development schedule is summarized in **Table 1** below for technical assessment purpose.

Proposed Development	
Total Site Area (m ²)	2,856
Paved Area (m ²)*	1,052

Table 1 - Key Development Parameters

* Please refer to Appendix B and Catchment Plan in Figure 4

3. Assessment Criteria

3.1.1 The Recommended Design Return Period based on Flood Level from SDM (Table 10) is adopted for this DIA. The recommendation is summarized in **Table 2** below.

Description	Design Return Periods
Intensively Used Agricultural Land	2 – 5 Years
Village Drainage Including Internal Drainage System under a polder Scheme	10 Years
Main Rural Catchment Drainage Channels	50 Years
Urban Drainage Trunk System	200 Years
Urban Drainage Branch System	50 Years

Table 2– Design Return Periods under SDM

3.1.2 The proposed village drainage system intended to collect runoff from the internal site and discharge to existing nearby public drainage system. 1 in 10 years return period is adopted for the drainage design.

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- 3.1.3 Stormwater drainage design will be carried out in accordance with the criteria set out in the Stormwater Drainage Manual published by DSD. The proposed design criteria to be adopted for design of this stormwater drainage system and factors which have been considered are summarised below.
 - 1. Intensity-Duration-Frequency Relationship The Recommended Intensity-Duration-Frequency relationship is used to estimate the intensity of rainfall. It can be expressed by the following algebraic equation.

$$i = \frac{a}{(t_d + b)^c}$$

The site is located within the HKO Headquarters Rainfall Zone. Therefore, for 10 years return period, the following values are adopted.

а	=	471.9
b	=	3.02
С	=	0.397

2. The peak runoff is calculated by the Rational Method i.e. $Q_p = 0.278 \text{CiA}$

where	Q_p	=	peak runoff in m ³ /s
	С	=	runoff coefficient (dimensionless)
	i	=	rainfall intensity in mm/hr
	А	=	catchment area in km ²

- 3. The run-off coefficient (C) of surface runoff are taken as follows:
 - Paved Area: C = 0.95
 Unpaved Area: C = 0.35

4. Manning's Equation is used for calculation of velocity of flow inside the channels:

Manning's Equation: $v = \frac{R^{\frac{1}{6}}}{n} R^{\frac{1}{2}} S_f^{\frac{1}{2}}$

Where,

V = velocity of the pipe flow (m/s)

 S_f = hydraulic gradient

n = manning's coefficient

R = hydraulic radius (m)

5. Colebrook-White Equation is used for calculation of velocity of flow inside the pipes:

Colebrook-White E	quatior	1:	$\underline{v} = -\sqrt{32gRS} \log \log \left(\frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{32gRS_f}}\right)$
where,	V S _f v D R	= = = =	velocity of the pipe flow (m/s) hydraulic gradient roughness value (m) kinematics viscosity of fluid pipe diameter (m) hydraulic radius (m)

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4. Proposed Drainage System

- 4.1.1 Proposed drainage system and existing asbuilt channels are designed/checked for collection of runoff from the application site and external catchment nearby. It is proposed to discharge to existing channel at Nam Hing West Road. The alignment, size and gradient of the proposed drains are shown in **Figure 3**. The catchment plan is shown in **Figure 4**.
- 4.1.2 The design calculations of proposed drains are shown in **Appendix A**.
- 4.1.3 The reference standard drawings of drains are shown in **Appendix C**.

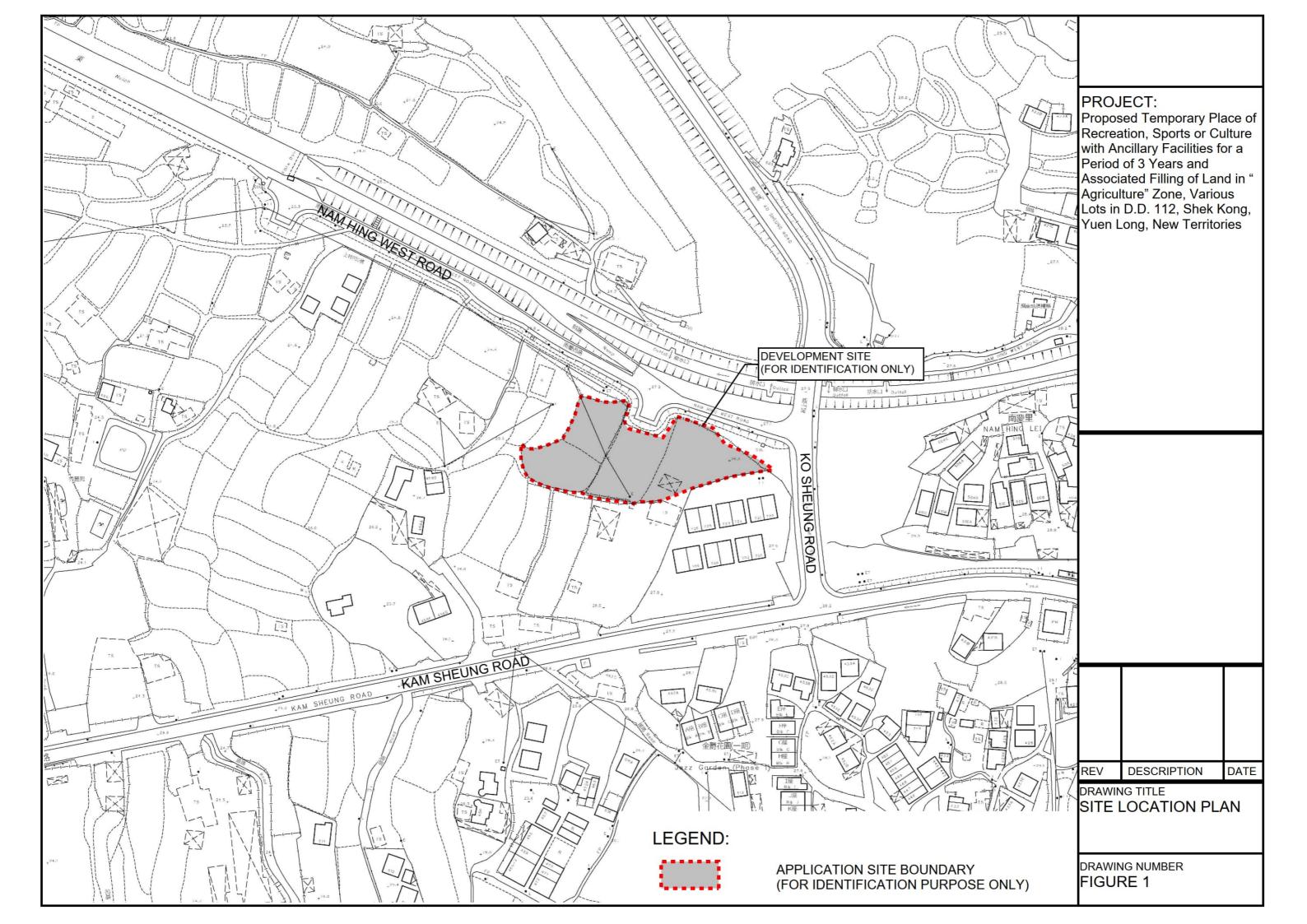
5. Conclusion

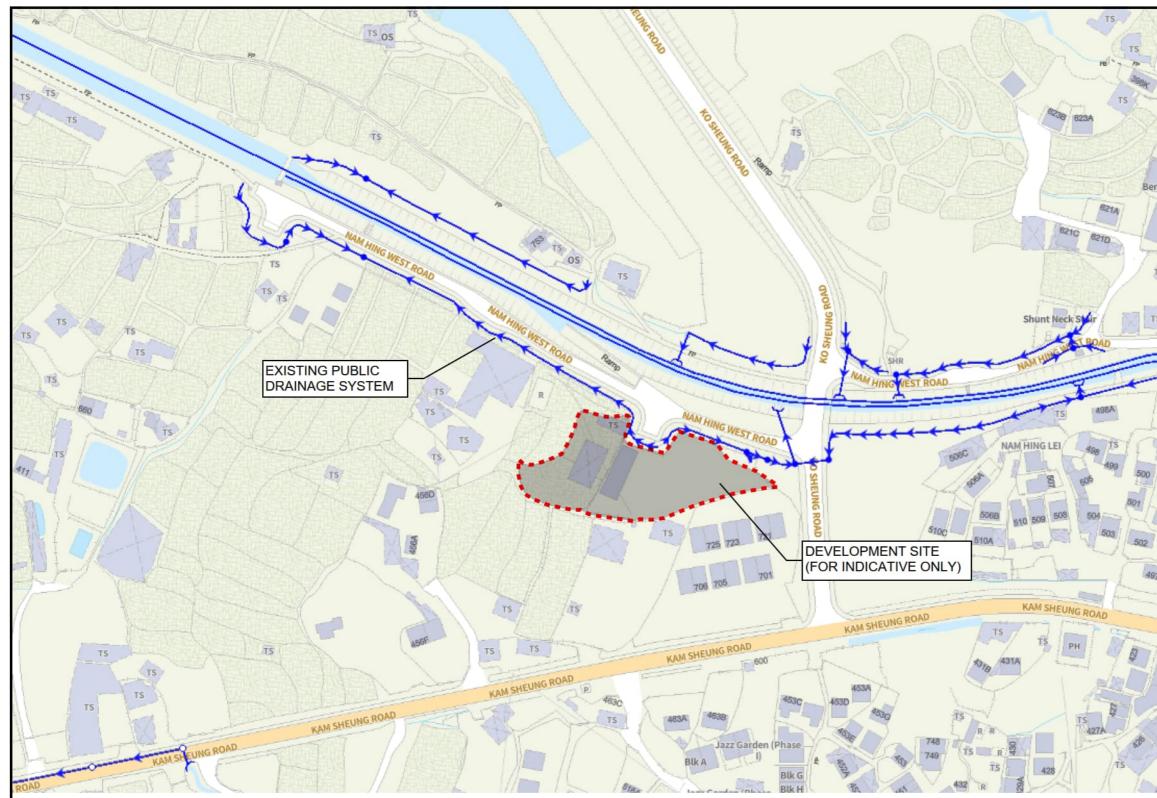
- 5.1.1 A drainage appraisal has been conducted for the Proposed Development. The surface runoff from the Application Site will be collected by the existing/proposed drains and discharged to the existing channel at Nam Hing West Road.
- 5.1.2 With the proposed drainage system, it is anticipated that there will be no significant drainage impact to the area after the implementation of the development.

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End of Text -

FIGURES



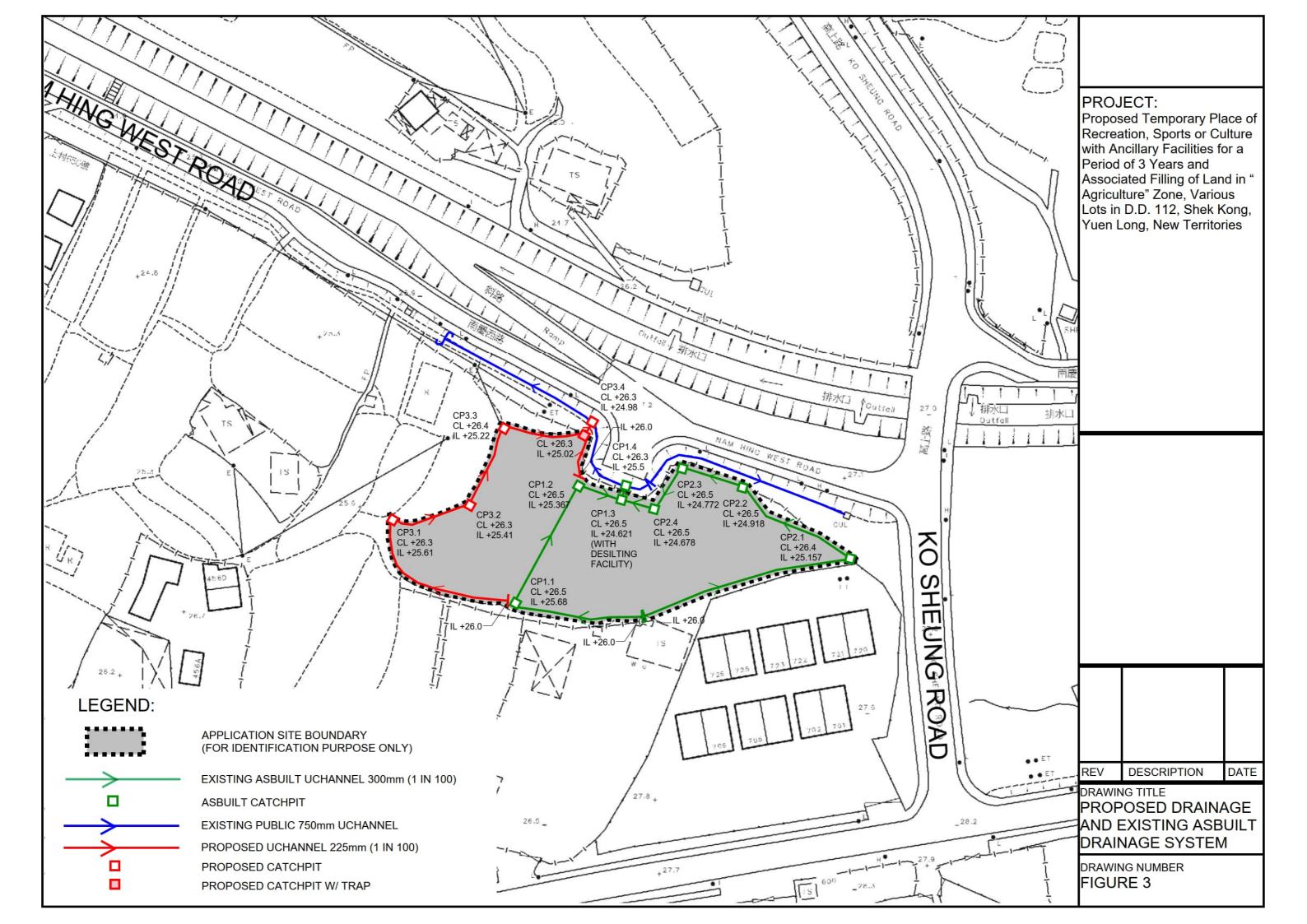


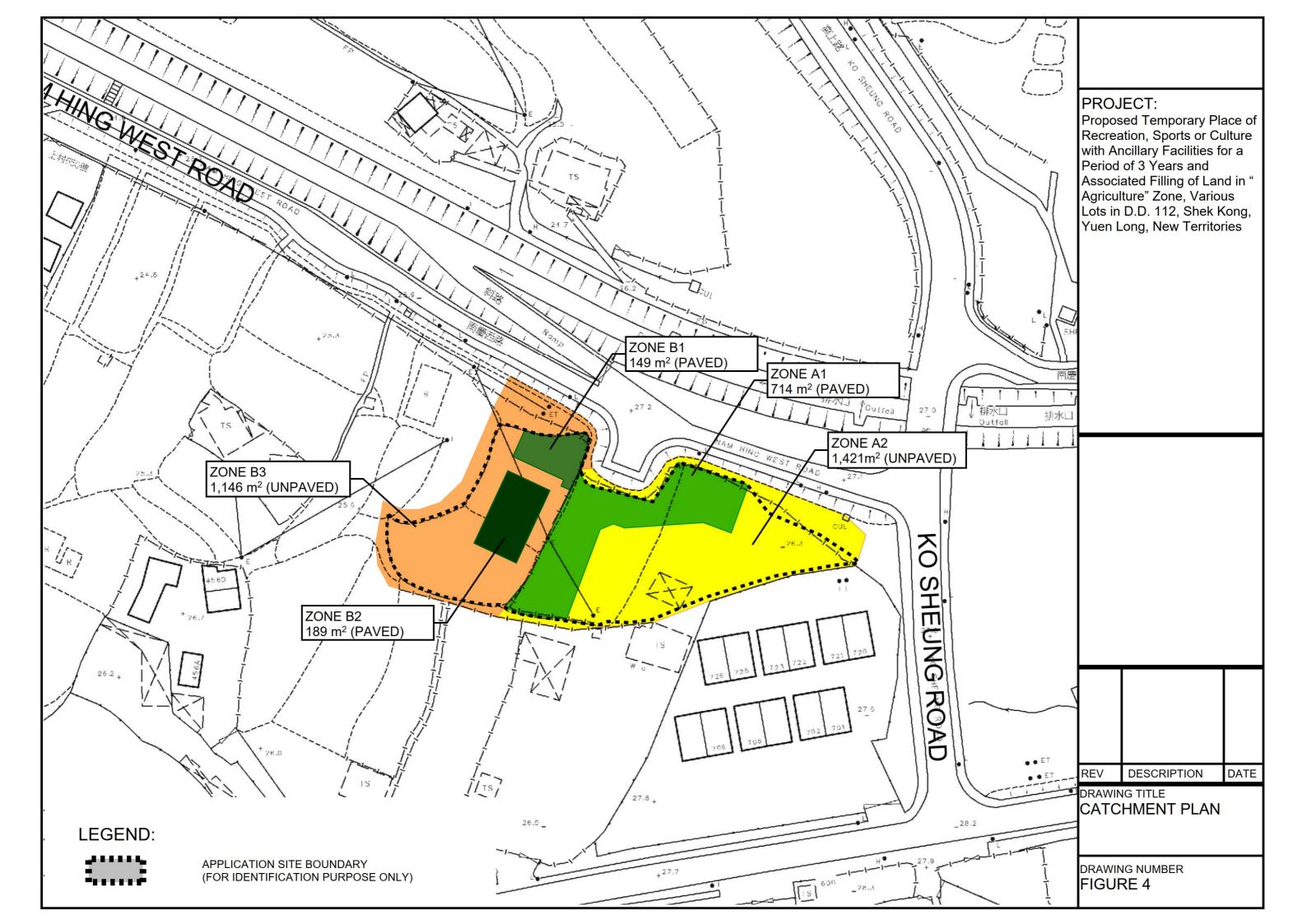
LEGEND:

	Combined Manhole
ъ	Overflow (Combined)
—	Pipe (Combined)
	Interface Valve Chamber
	Sewer Manhole
	Oil / Petrol Interceptor
ъ	Overflow (Sewer)
-	Pipe (Sewer)

н	Tapping Point (Sewer)	н	Tapping Point (Storm)
D	Sewer Terminal Manhole	0	Storm Water Terminal Manhole
•	Catchpit	7223	Tunnel Protection Zone (100m / 200m)
↦	Inlet	7223	Tunnel Protection Zone (General Range)
0	Storm Water Manhole		Tunnel / Box Culvert (Sewer)
+-(Outlet	1000	Tunnel / Box Culvert (Storm)
_	Pipe (Storm)		
-	Sand Trap		

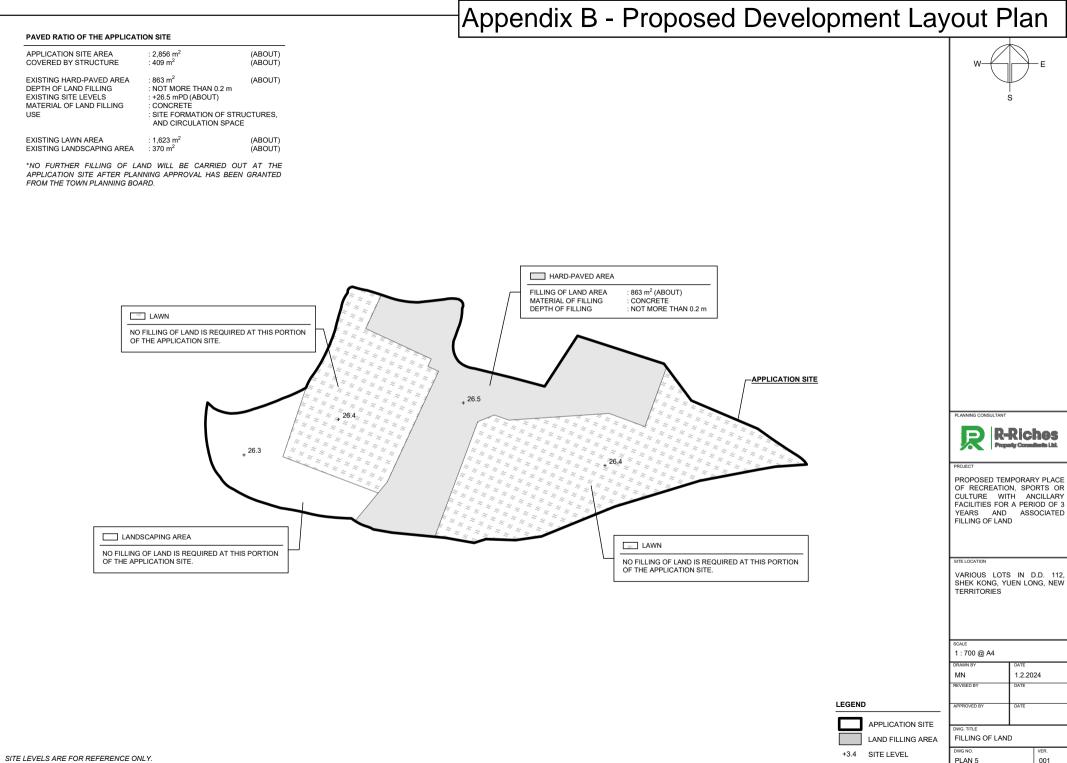
P) 388/389/ 632 633 633 633 633 633 633 633	PROJECT: Proposed Temporary Place of Recreation, Sports or Culture with Ancillary Facilities for a Period of 3 Years and Associated Filling of Land in " Agriculture" Zone, Various Lots in D.D. 112, Shek Kong, Yuen Long, New Territories
4988 398 397 4916 4916 492 497 496 495	
ZA 496A 495A KAM SHEUNG R TSE UK TSUEN	
	REV DESCRIPTION DATE DRAWING TITLE EXISTING DRAINAGE PLAN
	DRAWING NUMBER FIGURE 2



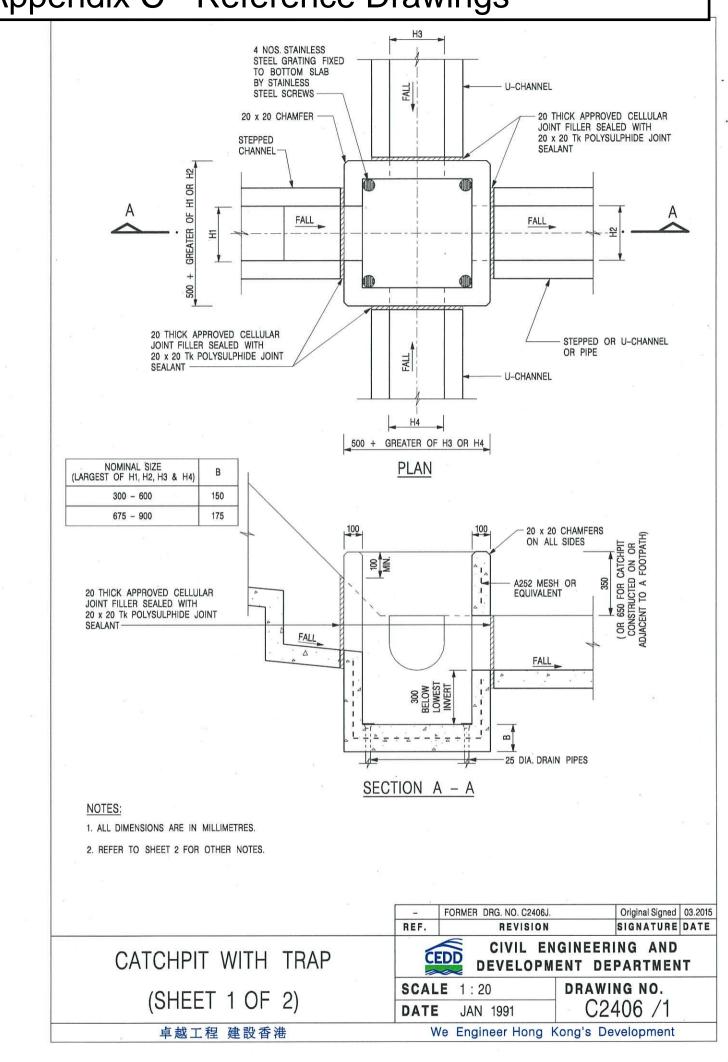


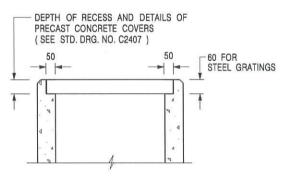
Appendix

U Channel 1 (Zone A1 + A	<u>2)</u>				
Runoff Estimation					
Design Return Period		1 in	10	years	
Paved Area	714 =		714	(m2)	
Unpaved Area	1421 =		1421	(m2)	. a
Total Equivalent Area	714 x 0.95 + 1421 x 0.35 =		1176	(m2)	$i = \frac{a}{(t_d + b)^c}$
Rainfall Intensity, I *			206	mm/hr	(-4)
Design Discharge Rate, Q	0.278 x 1176 x 206 / 1000000 =		0.067	m3/s	
U Channel				()	
Channel Size			300	(mm)	
Gradient		1 in	100		
Velocity			1.58	m/s	
Capacity			0.127	m3/s	
Utilization	0.067 / 0.127	=	53.12	%	OK
Othization		-			
		-			
U Channel 2 (Zone B1 + B Runoff Estimation		_			
U Channel 2 (Zone B1 + B Runoff Estimation Desian Return Period		_ 1 in	10	vears	
U Channel 2 (Zone B1 + B Runoff Estimation Design Return Period Paved Area	2 + B3) 338 =				
U Channel 2 (Zone B1 + B Runoff Estimation Design Return Period Paved Area Unpaved Area	2 + B3) 338 = 1146 =		10	vears (m2) (m2)	. a
U Channel 2 (Zone B1 + B Runoff Estimation Design Return Period Paved Area Unpaved Area Unpaved Area Total Equivalent Area	2 + B3) 338 =		10 338	vears (m2)	$\cdot i = \frac{a}{(t_1 + b)c}$
U Channel 2 (Zone B1 + B Runoff Estimation Design Return Period Paved Area Unpaved Area Total Equivalent Area Rainfall Intensity, 1 *	2 + B3) 338 = 1146 =		10 338 1146	vears (m2) (m2)	• $i = \frac{a}{(t_d + b)^c}$
U Channel 2 (Zone B1 + B Runoff Estimation Design Return Period Paved Area Unpaved Area Unpaved Area Total Equivalent Area	2 + B3) 338 = 1146 =		10 338 1146 722	vears (m2) (m2) (m2)	• $i = \frac{a}{(t_d + b)^c}$
U Channel 2 (Zone B1 + B Runoff Estimation Design Return Period Paved Area Unpaved Area Total Equivalent Area Rainfall Intensity, 1 *	2 + B3) 338 = 1146 = 338 × 0.95 + 1146 × 0.35 =		10 338 1146 722 206	vears (m2) (m2) (m2) mm/hr	• $i = \frac{a}{(t_d + b)^c}$
U Channel 2 (Zone B1 + B Runoff Estimation Design Return Period Paved Area Unpaved Area Total Equivalent Area Rainfall Intensity, 1 *	2 + B3) 338 = 1146 = 338 × 0.95 + 1146 × 0.35 =		10 338 1146 722 206	vears (m2) (m2) (m2) mm/hr	• $i = \frac{a}{(t_d + b)^c}$
U Channel 2 (Zone B1 + B Runoff Estimation Design Return Period Paved Area Unpaved Area Total Equivalent Area Rainfall Intensity, I * Design Discharge Rate, Q U Channel Channel Size	2 + B3) 338 = 1146 = 338 × 0.95 + 1146 × 0.35 =		10 338 1146 722 206	vears (m2) (m2) (m2) mm/hr	• $i = \frac{a}{(t_d + b)^c}$
U Channel 2 (Zone B1 + B Runoff Estimation Design Return Period Paved Area Unpaved Area Total Equivalent Area Rainfall Intensity, I * Design Discharge Rate, Q U Channel Channel Size Gradient	2 + B3) 338 = 1146 = 338 × 0.95 + 1146 × 0.35 =		10 338 1146 722 206 0.041	vears (m2) (m2) (m2) mm/hr m3/s	• $i = \frac{a}{(t_d + b)^c}$
U Channel 2 (Zone B1 + B Runoff Estimation Design Return Period Paved Area Unpaved Area Total Equivalent Area Rainfall Intensity, I * Design Discharge Rate, Q U Channel Channel Size	2 + B3) 338 = 1146 = 338 × 0.95 + 1146 × 0.35 =	1 in	10 338 1146 722 206 0.041 225	vears (m2) (m2) (m2) mm/hr m3/s	• $i = \frac{a}{(t_d + b)^c}$



Appendix C - Reference Drawings





ALTERNATIVE TOP SECTION FOR PRECAST CONCRETE COVERS / GRATINGS

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. ALL CONCRETE SHALL BE GRADE 20 /20.
- 3. CONCRETE SURFACE FINISH SHALL BE CLASS U2 OR F2 AS APPROPRIATE.
- 4. FOR DETAILS OF JOINT, REFER TO STD. DRG. NO. C2413.
- 5. CONCRETE TO BE COLOURED AS SPECIFIED.
- UNLESS REQUESTED BY THE MAINTENANCE PARTY AND AS DIRECTED BY THE ENGINEER, CATCHPIT WITH TRAP IS NORMALLY NOT PREFERRED DUE TO PONDING PROBLEM.
- 7. UPON THE REQUEST FROM MAINTENANCE PARTY, DRAIN PIPES AT CATCHPIT BASE CAN BE USED BUT THIS IS FOR CATCHPITS LOCATED AT SLOPE TOE ONLY AND AS DIRECTED BY THE ENGINEER.
- FOR CATCHPITS CONSTRUCTED ON OR ADJACENT TO A FOOTPATH, STEEL GRATINGS (SEE DETAIL 'A' ON STD. DRG. NO. C2405 /2) OR CONCRETE COVERS (SEE STD. DRG. NO. C2407) SHALL BE PROVIDED AS DIRECTED BY THE ENGINEER.
- 9. IF INSTRUCTED BY THE ENGINEER, HANDRAILING (SEE DETAIL 'J' ON STD. DRG. NO. C2405 /5; EXCEPT ON THE UPSLOPE SIDE) IN LIEU OF STEEL GRATINGS OR CONCRETE COVERS CAN BE ACCEPTED AS AN ALTERNATIVE SAFETY MEASURE FOR CATCHPITS NOT ON A FOOTPATH NOR ADJACENT TO IT. TOP OF THE HANDRAILING SHALL BE 1 000 mm MIN. MEASURED FROM THE ADJACENT GROUND LEVEL.
- 10. MINIMUM INTERNAL CATCHPIT WIDTH SHALL BE 1 000 mm FOR CATCHPITS WITH A HEIGHT EXCEEDING 1 000 mm MEASURED FROM THE INVERT LEVEL TO THE ADJACENT GROUND LEVEL. AND, STEP IRONS (SEE DSD STD. DRG. NO. DS1043) AT 300 c/c STAGGERED SHALL BE PROVIDED. THICKNESS OF CATCHPIT WALL FOR INSTALLATION OF STEP IRONS SHALL BE INCREASED TO 150 mm.
- 11. FOR RETROFITTING AN EXISTING CATCHPIT WITH STEEL GRATING, SEE DETAIL 'G' ON STD. DRG. NO. C2405 /4.
- 12. SUBJECT TO THE APPROVAL OF THE ENGINEER, OTHER MATERIALS CAN ALSO BE USED AS COVERS / GRATINGS.

	A	MINOR AMENDMENT.	Original Signed 04.2016
	- FORMER DRG. NO. C2406J.		Original Signed 03.2015
	REF.	REVISION	SIGNATURE DATE
CATCHPIT WITH TRAP	CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT		
(SHEET 2 OF 2)	SCAL	E 1:20 JAN 1991	drawing no. C2406 /2A
卓越工程 建設香港	We Engineer Hong Kong's Development		

